Numerical schemes for the hyperbolic Shallow-Water type model with two velocities

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Motivation



FIGURE - Floods in Vésubie, France 2020



 $\ensuremath{\mbox{Figure}}$ – Sediment transport and Deposition of the Rhone River

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- Presentation of the system
- In the homogenuous 1D Shallow water model with two velocities
 - Numerical schemes
 - Numerical results
- ID Shallow water model with two velocities with topography
- Onclusion and perspectives

The 2D shallow water type model with two velocities reads

$$\begin{cases} \partial_t h + \nabla \cdot (h\overline{U}) &= 0, \\ \partial_t (h\overline{U}) + \nabla \cdot (h(\overline{U} \otimes \overline{U} + \hat{U} \otimes \hat{U}) + \frac{g}{2} h^2 \mathbb{I}) &= 0, \\ \partial_t \hat{U} &+ (\overline{U} \cdot \nabla) \hat{U} + (\hat{U} \cdot \nabla) \overline{U} &= 0. \end{cases}$$
(SW₂)

The system is invariant by rotation so we study (SW_2) in the x-direction

$$\begin{array}{l} \partial_t h &+ \partial_x (h\overline{u}) &= 0\\ \partial_t (h\overline{u}) &+ \partial_x (h(\overline{u}^2 + \hat{u}^2) + \frac{g}{2}h^2) = 0\\ \partial_t \hat{u} &+ \partial_x (\overline{u}\hat{u}) &= 0 \end{array} \right\}$$
(1D)
$$\begin{array}{l} \partial_t (h\overline{v}) &+ \partial_x (h(\overline{u}\overline{v} + \hat{u}\hat{v})) = 0\\ \partial_t \hat{v} &+ \hat{u}\partial_x \overline{v} + \overline{u}\partial_x \hat{v} &= 0 \end{array} \right\}$$
with $\overline{U} = (\overline{u}, \overline{v})^t$ and $\hat{U} = (\hat{u}, \hat{v})^t.$

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Introduction

We consider the set of variables

$$U = (h, \overline{u}, \hat{u})$$

and the initial data for the Riemann problem

$$U(0,x) = \begin{cases} U_L = (h_L, \overline{u}_L, \hat{u}_L)^t \in \mathbb{R}^*_+ \times \mathbb{R}^2 & \text{if } x < 0, \\ U_R = (h_R, \overline{u}_R, \hat{u}_R)^t \in \mathbb{R}^*_+ \times \mathbb{R}^2 & \text{if } x \ge 0. \end{cases}$$

The eigenvalues are given by



Description of the Godunov-type schemes

Finite volume framework

- We consider a uniform discretization of the computational domain
- We denote $C_i =]x_{i-\frac{1}{2}}, x_{i+\frac{1}{2}}[$ the cell of length $\Delta x = x_{i+\frac{1}{2}} x_{i-\frac{1}{2}}$ and centered at x_i
- For any time tⁿ, we define tⁿ⁺¹ = tⁿ + Δtⁿ with Δtⁿ satisfying a CFL condition to be described later
- Let U_i^n be a piecewise constant approximation of U(x, t) at time t^n on the cell C_i
- We propose the following update formula

$$\forall i \in \mathbb{Z}, \forall n \in \mathbb{N} \quad U_i^{n+1} = U_i^n - \frac{\Delta t^n}{\Delta x} \left(\mathcal{F}_{i+\frac{1}{2}}^n - \mathcal{F}_{i-\frac{1}{2}}^n \right),$$

where

$$\mathcal{F}_{i+\frac{1}{2}}^{n} \approx \frac{1}{\Delta t^{n}} \int_{t^{n}}^{t^{n+1}} F\left(U\left(t, x_{i+\frac{1}{2}}\right)\right) dt.$$

The initialization of the algorithm can be computed with

$$\forall i \in \mathbb{Z} \quad U_i^0 = \frac{1}{\Delta x} \int_{x_{i-\frac{1}{2}}}^{x_{i+\frac{1}{2}}} U(x,0) dx.$$

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Godunov method

Godunov observed that U_i^n define at each cell interface $x_{i+\frac{1}{2}}$ a Riemann problem

$$\begin{cases} \partial_t U + \partial_x F(U) &= 0 \\ U(t^n, x) &= \begin{cases} U_L & \text{if } x < x_{i+\frac{1}{2}} \\ U_R & \text{if } x \ge x_{i+\frac{1}{2}} \end{cases} \end{cases}$$



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The considered approximated Riemann solvers are

$$\tilde{U}\left(\frac{x}{t}, U_L, U_R\right) = \begin{cases} U_L = U_{\frac{1}{2}} & \text{if } \frac{x}{t} < \lambda_1, \\ \tilde{U}_{j+\frac{1}{2}} & \text{if } \lambda_j < \frac{x}{t} < \lambda_{j+1} \text{ for } j = 1, ..., N-1, \\ U_R = U_{N+\frac{1}{2}} & \text{if } \frac{x}{t} > \lambda_N. \end{cases}$$

The update at time t^{n+1} is then defined by

$$U_i^{n+1} = \frac{1}{\Delta x} \int_0^{\frac{\Delta x}{2}} \tilde{U}\left(\frac{x}{\Delta t^n}, U_{i-1}^n, U_i^n\right) dx + \frac{1}{\Delta x} \int_{-\frac{\Delta x}{2}}^0 \tilde{U}\left(\frac{x}{\Delta t^n}, U_i^n, U_{i+1}^n\right) dx.$$

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Description of the Godunov-type schemes

• The external waves of the approximated solution have to be faster than the external wave speed of the exact solution

$$\lambda_{L} = \min \left(\overline{u}_{L} - c_{L}, \overline{u}_{R} - c_{R} \right), \lambda_{R} = \max \left(\overline{u}_{L} + c_{L}, \overline{u}_{R} + c_{R} \right),$$

where $c_X = \sqrt{gh_X + 3\hat{u}_X^2}$.

In time step has to satisfy the following CFL condition

$$(\max_{j,i}|\lambda_{j,i+\frac{1}{2}}^n|)\Delta t^n \leq \frac{\Delta x}{2},$$

where $\lambda_{j,i+\frac{1}{2}}^n = \lambda_j(U_i^n, U_{i+1}^n).$

 The scheme has to satisfy a consistency property in the sense Harten and Lax showed in (Lax 83)

$$\frac{1}{\Delta x}\int_{-\frac{\Delta x}{2}}^{\frac{\Delta x}{2}}\tilde{U}(\frac{x}{\Delta t^{n}},U_{L}U_{R})dx=\frac{1}{\Delta x}\int_{-\frac{\Delta x}{2}}^{\frac{\Delta x}{2}}U_{r}(\frac{x}{\Delta t^{n}},U_{L},U_{R})dx,$$

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The consistency with the integral form of the conservation law leads to the following intermediate states

$$\begin{cases} h_{HLL} = \frac{[h(\lambda - \overline{u})]}{[\lambda]}, \\ h_{HLL}\overline{u}_{HLL} = \frac{[\lambda h\overline{u} - h(\overline{u}^2 + \hat{u}^2) - \frac{g}{2}h^2]}{[\lambda]}, \\ \hat{u}_{HLL} = \frac{[\hat{u}(\lambda - \overline{u})]}{[\lambda]}. \end{cases}$$

Assume $h_L > 0$ or $h_R > 0$ then, the intermediate state h_{HLL} is positive.

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 FIGURE – Two rarefactions case. Plots of the variables using HLL solver for 1000 grid

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The HLL* scheme

• The quantity $\frac{\hat{u}}{h}$ jumps only along the intermediate contact discontinuity. In fact, from (SW_2) , we can deduce that for regular solutions

$$\partial_t(\frac{\hat{u}}{h}) + \overline{u}\partial_x(\frac{\hat{u}}{h}) = 0.$$

- The *HLL* scheme is used to update only the classical shallow water variables (h, \overline{u}) and to compute the interface mass and momentum fluxes \mathcal{F}_{HLL}^h and $\mathcal{F}_{HLL}^{h\overline{u}}$.
- The shear velocity \hat{u} is updated using an upwind strategy

$$\mathcal{F}^{\hat{u}}_{\{\text{HLL}, up, i+\frac{1}{2}\}} = \frac{\hat{u}^{n}_{i}}{h^{n}_{i}} \mathcal{F}^{h+}_{\{\text{HLL}, i+\frac{1}{2}\}} + \frac{\hat{u}^{n}_{i+1}}{h^{n}_{i+1}} \mathcal{F}^{h-}_{\{\text{HLL}, i+\frac{1}{2}\}},$$

1D Shallow water model with two velocities

The HLL* solver can also be seen as a 3- waves approximate Riemman solver

$$\tilde{U}_{HLL^*}(\frac{x}{t}, U_L, U_R) = \begin{cases} U_L & \text{if } \frac{x}{t} < \lambda_L, \\ U_L^* & \text{if } \lambda_L < \frac{x}{t} < \tilde{\lambda}, \\ U_R^* & \text{if } \tilde{\lambda} < \frac{x}{t} < \lambda_R, \\ U_R & \text{if } \frac{x}{t} > \lambda_R. \end{cases}$$



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To construct the numerical scheme

- we consider the consistency relations
- **2** we impose the continuity of *h* and \overline{u} through the $\tilde{\lambda}$ -wave

$$h_L^* = h_R^*$$
 and $\overline{u}_L^* = \overline{u}_R^*$.

• we impose the continuity of $\frac{\hat{u}}{h}$ on the external waves λ_L and λ_R

$$\frac{\hat{u}_L}{h_L} = \frac{\hat{u}_L^*}{h_L^*} \qquad \text{and} \qquad \frac{\hat{u}_R}{h_R} = \frac{\hat{u}_R^*}{h_R^*}$$

1D Shallow water model with two velocities

For $\frac{\hat{u}_R}{h_R} \neq \frac{\hat{u}_L}{h_L}$, the intermediate states are

$$\begin{cases} h_{L}^{*} = h_{R}^{*} = h_{HLL}, \\ \overline{u}_{L}^{*} = \overline{u}_{R}^{*} = \overline{u}_{HLL}, \\ \hat{u}_{L}^{*} = \hat{u}_{L} \frac{h_{HLL}}{h_{L}}, \\ \hat{u}_{R}^{*} = \hat{u}_{R} \frac{h_{HLL}}{h_{R}}, \\ \tilde{\lambda} = \lambda_{R} - \frac{h_{R} (\lambda_{R} - \overline{u}_{R})}{h_{HLL}}. \end{cases}$$
(HLL*)

If $\frac{\hat{u}_R}{h_R} = \frac{\hat{u}_L}{h_L}$, we get $\hat{u}_L^* = \hat{u}_R^* = \hat{u}_{HLL}$ and we choose $\tilde{\lambda}$ defined above. In addition, we have

- $\lambda_L < \tilde{\lambda} < \lambda_R$ • $sgn(\mathcal{F}^h_{\{HLL\}}) = sgn(\tilde{\lambda})$
- $\mathcal{F}^{\hat{u}}_{\{HLL,up\}} = \mathcal{F}^{\hat{u}}_{HLL^*}$

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 FIGURE – Two rarefactions case. Plots of the variables using HLL and HLL* solvers for

 1000 grid cells

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 FIGURE – Dam break problem. Plots of the variables using HLL and HLL* solvers for

 1000 grid cells

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The unknowns are

$$\begin{cases} U_L^* = (h_L^*, \overline{u}_L^*, \hat{u}_L^*) \\ U_R^* = (h_R^*, \overline{u}_R^*, \hat{u}_R^*) \\ \end{cases} + \lambda^* \Rightarrow 7 \text{ relations.} \end{cases}$$

The 3-waves HLLC type scheme

Rankine-Hugoniot relations

• $h_L^* \overline{u}_L^* - h_L \overline{u}_L = \lambda_L (h_L^* - h_L)$ on λ_L ,

•
$$h_R \overline{u}_R - h_R^* \overline{u}_R^* = \lambda_R (h_R - h_R^*)$$
 on λ_R

• $\overline{u}_{L}^{*} = \overline{u}_{R}^{*} = \lambda^{*}$, • $\frac{\partial_{L}}{h_{L}} = \frac{\partial_{L}^{*}}{h_{L}^{*}}$ on λ_{L} , • $\frac{\partial_{R}}{h_{R}} = \frac{\partial_{R}^{*}}{h_{R}^{*}}$ on λ_{R} .

The consistency relations of the discharge

$$\lambda_R h_R^* \overline{u}_R^* - \lambda_L h_L^* \overline{u}_L^* + \lambda^* \left(h_L^* \overline{u}_L^* - h_R^* \overline{u}_R^* \right) = \left(\lambda_R - \lambda_L \right) h_{HLL} \overline{u}_{HLL}.$$

The 3-waves HLLC type scheme can be solved to obtain

$$h_{L}^{*} = h_{L} \left(\frac{\lambda_{L} - \overline{u}_{L}}{\lambda_{L} - \overline{u}_{HLL}} \right),$$

$$h_{R}^{*} = h_{R} \left(\frac{\lambda_{R} - \overline{u}_{R}}{\lambda_{R} - \overline{u}_{HLL}} \right),$$

$$\overline{u}_{L}^{*} = \overline{u}_{HLL},$$

$$\overline{u}_{R}^{*} = \overline{u}_{HLL},$$

$$\hat{u}_{L}^{*} = \hat{u}_{L} \left(\frac{\lambda_{L} - \overline{u}_{L}}{\lambda_{L} - \overline{u}_{HLL}} \right),$$

$$\hat{u}_{R}^{*} = \hat{u}_{R} \left(\frac{\lambda_{R} - \overline{u}_{R}}{\lambda_{R} - \overline{u}_{HLL}} \right),$$

$$\lambda^{*} = \overline{u}_{HLL}.$$
(HLLC3)

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For $h_L > 0$ or $h_R > 0$, we are able to prove that

- $\lambda_L < \overline{u}_{HLL} < \lambda_R$
- $h_L^* > 0$ and $h_R^* > 0$.

Numerical results



Numerical results



FIGURE – The Dam break problem : Convergence of L^2 norm errors at t = 0.05s with mesh refinement for h on the left, for \overline{u} in the middle and for \hat{u} on the right.

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1D Shallow water model with two velocities and topography

The model reads

$$\begin{cases} \partial_t h &+ \partial_x (h\overline{u}) &= 0, \\ \partial_t (h\overline{u}) &+ \partial_x (h(\overline{u}^2 + \hat{u}^2) + \frac{g}{2}h^2) &= -gh\partial_x Z, \\ \partial_t \hat{u} &+ \partial_x (\overline{u}\hat{u}) &= 0. \end{cases}$$
 (SW₂)

- The system has an additional eigenvalue $\lambda_0 = 0$
- The exact solution of the integral of the topography is very difficult to compute

Main Objective

To develop a "Well balanced " scheme based on the analysis of the Riemann problem

- able to excatly recover any steady solution in 1D over an arbitrary topography
- preserves the non-negativity of the water heights

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Numerical schemes



- Using strategy done in (Dansac 16) and corrected in (Mbaye 21), we construct a numerical scheme named *HLLC*₀
- A new numerical scheme named $HLLC_0^*$ (in the same way of HLL*) is introduced



Total head and discharge \mathcal{L}^2 errors for the subcritical solution

	Total head	Discharge
HLLC ₀	8.943e-11	2.46e-13
HLLC [*]	8.942e-11	1.63e-13

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The 4-waves *HLLC* type scheme named $HLLC_{\overline{u},0}$



- we have 3 intermediates states
- we have 10 unknowns
- we have to preserve the positivity of the water heights
- we have to assure that $\lambda_L < \lambda^* < \lambda_R$ and that λ^* keeps its sign

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Conclusions

- Proposed several numerical schemes for the resolution of the homogenuous (SW_2)
- Proposed briefly several numerical schemes for the resolution of the (SW_2) with the topography

Perspectives

- To construct the piecewise C^1 steady solutions of (SW_2)
- To complete the $HLLC_{\overline{u},0}$ Riemann solver
- To do the numerical scheme for the 2D shallow water model with two velocities
- To extend the numerical scheme for the shear shallow water model

Thankyou !

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